

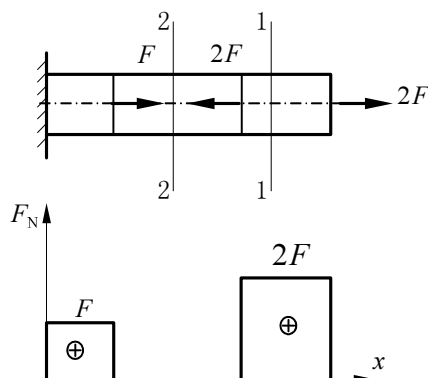
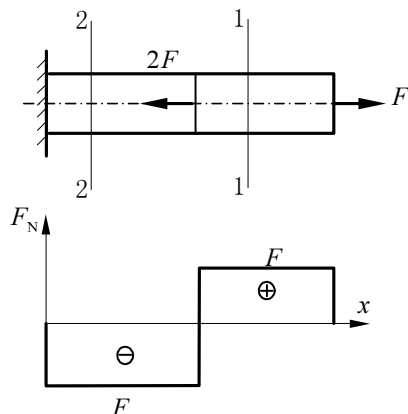
第 1 章 绪论及基本概念（无习题）

第二章 轴向拉伸和压缩

2-1 试求图示各杆 1-1 和 2-2 横截面上的轴力，并作轴力图。

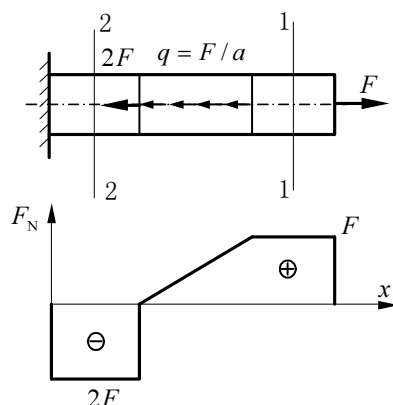
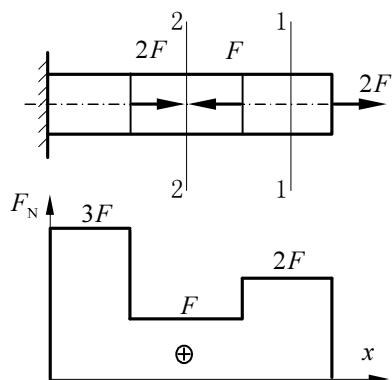
(a) 解: $F_{N1} = +F$; $F_{N2} = -F$;

(b) 解: $F_{N1} = +2F$; $F_{N2} = 0$;



(c) 解: $F_{N1} = +2F$; $F_{N2} = +F$ 。

(d) 解: $F_{N1} = F, F_{N2} = -2F$ 。



2-2 试求图示等直杆横截面 1-1, 2-2 和 3-3 上的轴力，并作轴力图。若横截面面积 $A = 400 \text{ mm}^2$ ，试求各横截面上的应力。

解: $F_{N1} = -20 \text{ kN}$

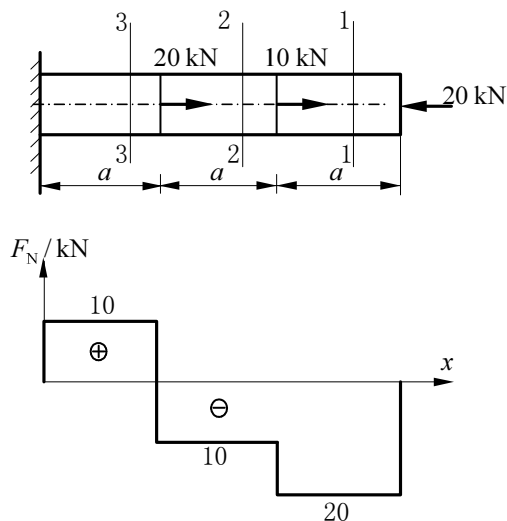
$F_{N2} = -10 \text{ kN}$

$F_{N3} = +10 \text{ kN}$

$$\sigma_1 = \frac{F_{N1}}{A} = \frac{-20 \times 10^3}{400 \times 10^{-6}} = -50 \text{ MPa}$$

$$\sigma_2 = \frac{F_{N2}}{A} = \frac{-10 \times 10^3}{400 \times 10^{-6}} = -25 \text{ MPa}$$

$$\sigma_3 = \frac{F_{N3}}{A} = \frac{10 \times 10^3}{400 \times 10^{-6}} = +25 \text{ MPa}$$



$$F_{N_3} = +10 \text{ kN}$$

$$\sigma_1 = \frac{F_{N1}}{A_1} = \frac{-20 \times 10^3}{200 \times 10^{-6}} = -100 \text{ MPa}$$

$$\sigma_2 = \frac{F_{N2}}{A_2} = \frac{-10 \times 10^3}{300 \times 10^{-6}} = -33.3 \text{ MPa}$$

$$\sigma_3 = \frac{F_{N3}}{A_3} = \frac{10 \times 10^3}{400 \times 10^{-6}} = +25.0 \text{ MPa}$$

1) 求内力

取 I-I 分离体 $\sum M_c = 0$

$$q \times \frac{(4.37 + 4.5)^2}{2} - F_{RA}(4.37 + 4.5) + F_{EG} \times 2.2 = 0$$

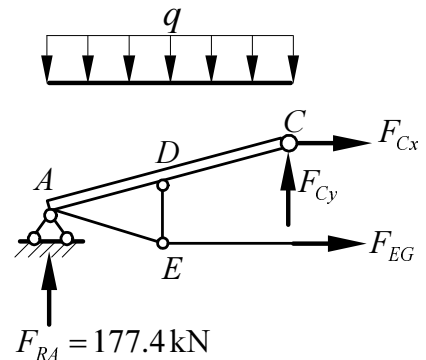
取节点 E 为分离体

$$\sum F_x = 0, \quad F_{AE} \cos \alpha = 356 \text{ kN}$$

$$\overline{AE} = \sqrt{4.37^2 + 1^2} = 4.47 \text{ m}$$

$$\cos \alpha = \frac{4.37}{4.47}$$

$$\text{故 } F_{AE} = \frac{356}{\cos \alpha} = \frac{356 \times 4.47}{4.37} = 366 \text{ kN (拉)}$$



75×8 等边角钢的面积 $A=11.5 \text{ cm}^2$

$$\sigma_{EG} = \frac{F_{EG}}{2A} = \frac{356 \times 10^3}{2 \times 11.5 \times 10^{-4}} = 155 \text{ MPa (拉)}$$

$$\sigma_{AE} = \frac{F_{AE}}{2A} = \frac{366 \times 10^3}{2 \times 11.5 \times 10^{-4}} = 159 \text{ MPa} \quad (\text{拉})$$

2-5 石砌桥墩的墩身高 $l = 10\text{m}$ ，其横截面尺寸如图所示。若荷载 $F = 1000\text{kN}$ ，材料的密度 $\rho = 2.35 \times 10^3 \text{ kg/m}^3$ 求墩身底部横截面上的压应力。

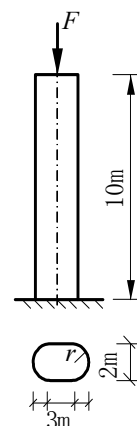
解：墩横截面积 A

$$A = 3 \times 2 + \frac{\pi \times 2^2}{4} = 9.14 \text{ m}^2,$$

$$\sigma_{\text{底}} = \frac{F}{A} + \frac{\rho g A l}{A} = \frac{F}{A} + 10 \rho g$$

$$= \frac{1000 \times 10^3}{9.14} + 10 \times 2.35 \times 10^3 \times 9.8$$

$$= 0.34 \text{ MPa} \quad (\text{压})$$



2-6 图示拉杆承受轴向拉力 $F = 10\text{kN}$ ，杆的横截面面积 $A = 100\text{mm}^2$ 。如以 α 表示斜截面与横截面的夹角，试求当 $\alpha = 0^\circ, 30^\circ, 45^\circ, 60^\circ, 90^\circ$ 时各斜截面上的正应力和切应力，并用图表示其方向。

解： $\sigma_\alpha = \sigma_0 \cos^2 \alpha$

$$\tau_\alpha = \frac{\sigma_0}{2} \sin 2\alpha$$

$$\sigma_{0^\circ} = \frac{F}{A} = \frac{10 \times 10^3}{100 \times 10^{-6}} = 100 \text{ MPa}$$

$$\tau_{0^\circ} = 0$$

$$\sigma_{30^\circ} = 100 \cos^2 30^\circ = 100 \times \left(\frac{\sqrt{3}}{2}\right)^2 = 75 \text{ MPa}$$

$$\tau_{30^\circ} = \frac{100}{2} \sin 2 \times 30^\circ = 43.2 \text{ MPa}$$

$$\sigma_{45^\circ} = 100 \cos^2 45^\circ = 100 \times \left(\frac{\sqrt{2}}{2}\right)^2 = 50 \text{ MPa}$$

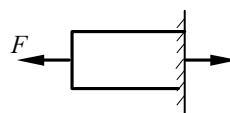
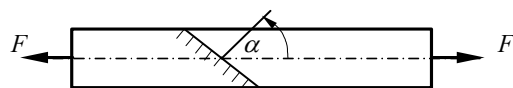
$$\tau_{45^\circ} = \frac{100}{2} \sin 2 \times 45^\circ = 50 \text{ MPa}$$

$$\sigma_{60^\circ} = 100 \cos^2 60^\circ = 100 \times \left(\frac{1}{2}\right)^2 = 25 \text{ MPa}$$

$$\tau_{60^\circ} = \frac{100}{2} \sin 2 \times 60^\circ = \frac{100}{2} \times \frac{\sqrt{3}}{2} = 43.3 \text{ MPa}$$

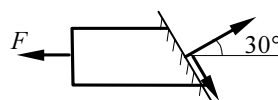
$$\sigma_{90^\circ} = 0$$

$$\tau_{90^\circ} = \frac{100}{2} \sin 2 \times 90^\circ = 0$$



$$\sigma_0 = 100 \text{ MPa}$$

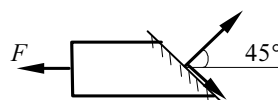
$$\alpha = 0^\circ$$



$$\sigma_{30^\circ} = 75 \text{ MPa}$$

$$\alpha = 30^\circ$$

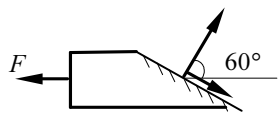
$$\tau_{30^\circ} = 43.2 \text{ MPa}$$



$$\sigma_{45^\circ} = 50 \text{ MPa}$$

$$\alpha = 45^\circ$$

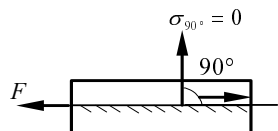
$$\tau_{45^\circ} = 50 \text{ MPa}$$



$$\sigma_{60^\circ} = 25 \text{ MPa}$$

$$\alpha = 60^\circ$$

$$\tau_{60^\circ} = 43.3 \text{ MPa}$$



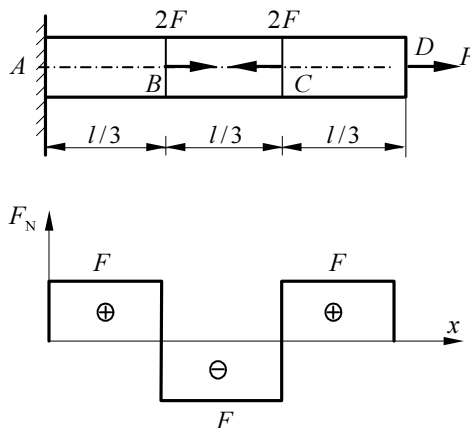
$$\alpha = 90^\circ$$

$$\tau_{90^\circ} = 0$$

2-7 一根等直杆受力如图所示。已知杆的横截面积 A 和材料的弹性模量 E 。试作轴力图，并求杆端点 D 的位移。

解：

$$\begin{aligned}\Delta_D &= \sum \frac{F_N l}{EA} \\ &= 2 \frac{F \cdot l/3}{EA} - \frac{F \cdot l/3}{EA} \\ &= \frac{Fl}{3EA}\end{aligned}$$



2-8 一木桩柱受力如图所示。柱的横截面为边长 200mm 的正方形，材料可认为符合胡克定律，其弹性模量 $E=10 \text{ GPa}$ 。如不计柱的自重，试求：

- (1) 作轴力图；
- (2) 各段柱横截面上的应力；
- (3) 各段柱的纵向线应变；
- (4) 柱的总变形。

解： $\sigma_{AC} = \frac{100 \times 10^3}{200 \times 200 \times 10^{-6}} = 2.5 \text{ MPa}$ (压)

$$\sigma_{CB} = \frac{260 \times 10^3}{200 \times 200 \times 10^{-6}} = 6.5 \text{ MPa} \quad (\text{压})$$

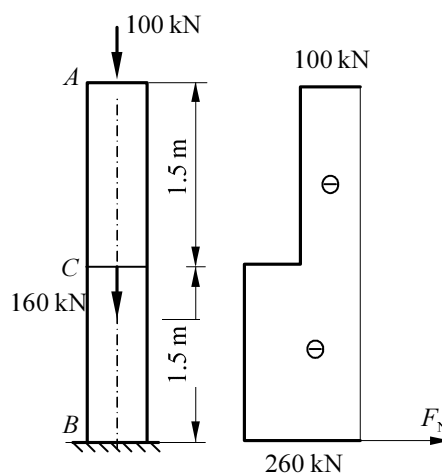
$$\begin{aligned}\Delta l_{AC} &= \frac{-F_{NAC} l_{AC}}{EA} = \frac{-100 \times 10^3 \times 1.5}{10 \times 10^9 \times 40000 \times 10^{-6}} \\ &= -0.375 \text{ mm}\end{aligned}$$

$$\Delta l_{CB} = \frac{-F_{NCB} l_{CB}}{EA} = \frac{-260 \times 10^3 \times 1.5}{10 \times 10^9 \times 40000 \times 10^{-6}} = -0.975 \text{ mm}$$

$$\Delta l = -\Delta l_{AC} - \Delta l_{CB} = -0.375 - 0.975 = -1.35 \text{ mm}$$

$$\varepsilon_{AC} = \frac{\sigma_{AC}}{E} = \frac{-2.5 \times 10^6}{10 \times 10^9} = -0.25 \times 10^{-3}$$

$$\varepsilon_{CB} = \frac{\sigma_{CB}}{E} = \frac{-6.5 \times 10^6}{10 \times 10^9} = -0.65 \times 10^{-3}$$



2-9 一根直径 $d = 16 \text{ mm}$ 、长 $l = 3 \text{ m}$ 的圆截面杆，承受轴向拉力 $F = 30 \text{ kN}$ ，其伸长为 $\Delta l = 2.2 \text{ mm}$ 。试求杆横截面上的应力与材料的弹性模量 E 。

解： $\sigma = \frac{F}{A} = \frac{30 \times 10^3}{\frac{\pi \times 16^2}{4} \times 10^{-6}} = 149 \text{ MPa}$

$$E = \frac{\sigma l}{\Delta l} = \frac{149 \times 10^6 \times 3}{2.2 \times 10^{-3}} = 203 \text{ GPa}$$

2-10 (1) 试证明受轴向拉伸(压缩)的圆截面杆横截面沿圆周方向的线应变 ε_s 。等于直径方向的线应变 ε_d 。

(2) 一根直径为 $d = 10 \text{ mm}$ 的圆截面杆，在轴向拉力 F 作用下，直径减小 0.0025 mm 。如材料的弹性模量 $E = 210 \text{ GPa}$ ，泊松比 $\nu = 0.3$ ，试求轴向拉力 F 。

(3) 空心圆截面钢杆，外直径 $D = 120 \text{ mm}$ ，内直径 $d = 60 \text{ mm}$ ，材料的泊松比 $\nu = 0.3$ 。当其受轴向拉伸时，已知纵向线应变 $\varepsilon = 0.001$ ，试求其壁厚 δ 。

解：(1) 证明 $\varepsilon_s = \varepsilon_d$

圆截面原圆周长 $s = \pi d$

变形后圆周长 $s' = \pi (d + \Delta d)$

$$\varepsilon_s = \frac{s' - s}{s} = \frac{\pi (d + \Delta d) - \pi d}{\pi d} = \frac{\Delta d}{d} = \varepsilon_d$$

(2) 求轴向拉力 F

$$\text{横向应变 } \varepsilon' = \frac{\Delta d}{d} = \frac{0.0025}{10} = 0.00025$$

$$\text{纵向应变 } \varepsilon = \frac{\varepsilon'}{\nu} = \frac{0.00025}{0.3} = 0.00083$$

$$F = EA\varepsilon = 210 \times 10^9 \times \frac{\pi \times 10^2}{4 \times 10^6} \times 0.00083 = 13.75 \text{ kN}$$

(3) 求变形后的壁厚 δ

$$\varepsilon' = \varepsilon \nu = 0.001 \times 0.25 = 0.00025$$

$$\text{则变形后的壁厚 } \delta = \frac{D - d}{2} - \varepsilon' \left(\frac{D - d}{2} \right)$$

$$\delta = (1 - \varepsilon') \frac{D - d}{2} = (1 - 0.00025) \times 30 = 29.99 \text{ mm}$$

2-11 受轴向拉力 F 作用的箱形薄壁杆如图所示。已知该杆材料的弹性常数为 E ， ν ，试求 C 与 D 两点间的距离改变量 Δ_{CD} 。

解： $\sigma_z = \frac{F}{A} = \frac{F}{(a + \delta)^2 - (a - \delta)^2} = \frac{F}{4a\delta}$

$$\varepsilon_x = \varepsilon_y = -\nu \frac{\sigma_z}{E} = -\frac{\nu F}{4a\delta E}$$

$$\Delta_{Dx} = \frac{2}{3} a \varepsilon_x = \frac{-\nu F a}{6a\delta E} = \frac{-\nu F}{6\delta E}$$

$$\Delta_{Cy} = \frac{3}{4} a \varepsilon_y = \frac{-3\nu F}{16\delta E}$$

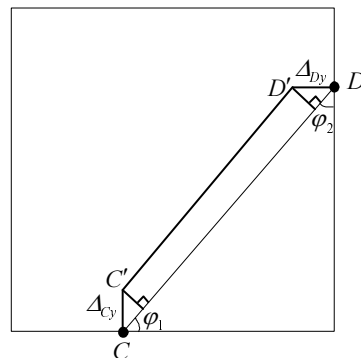
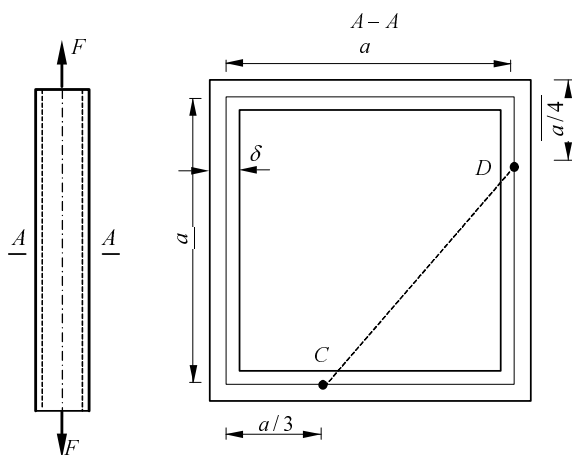
$$\Delta_{CD} = \Delta_{Dx} \cos \varphi_1 + \Delta_{Cy} \cos \varphi_2 = \frac{-\nu F}{4\delta E} \left(\frac{2}{3} \cos \varphi_1 + \frac{3}{4} \cos \varphi_2 \right) \quad (1)$$

其中 $\cos \varphi_1 = \frac{\frac{2}{3}}{\frac{\sqrt{145}}{12}} = \frac{8}{\sqrt{145}}$ ， $\varphi_1 = 48.37^\circ$

$$\cos \varphi_2 = \frac{\frac{3}{4}}{\frac{\sqrt{145}}{12}} = \frac{9}{\sqrt{145}}$$

代入式 (1)，得 $\Delta_{CD} = -1.003 \frac{\nu F}{4\delta E}$

注：图 2-11a 中， $C'D'$ 实际上与 CD 不平行，但因是小变形，且 φ_1, φ_2 相差不大，故取图示



近似计算。

2-12 图示结构中, AB 为水平放置的刚性杆, 杆 1, 2, 3 材料相同, 其弹性模量 $E=210\text{GPa}$, 已知 $l=1\text{m}$, $A_1=A_2=100\text{mm}^2$, $A_3=150\text{mm}^2$, $F=20\text{kN}$ 。试求 C 点的水平位移和铅垂位移。

解: (1) 受力图 (a)

$$\sum F_x = 0, F_3 = 0, F_2 = F_1 = \frac{F}{2}.$$

(2) 变形协调图 (b)

因 $F_3 = 0$, 故 $\Delta l_3 = 0$

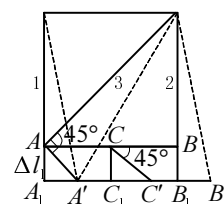
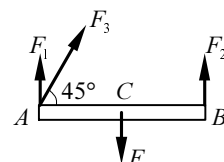
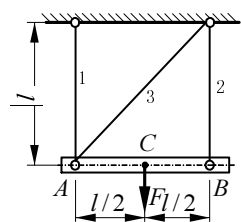
$$\begin{aligned}\Delta l_1 &= \frac{F_1 \cdot l}{EA_1} = \frac{\frac{F}{2} l}{EA_1} = \frac{10 \times 10^3 \times 1}{210 \times 10^9 \times 100 \times 10^{-6}} \\ &= \frac{1}{2100} \text{m} = 0.476 \text{mm} \quad (\text{向下})\end{aligned}$$

$$\Delta l_2 = \Delta l_1 = 0.476 \text{mm} \quad (\text{向下})$$

为保证 $\Delta l_3 = 0$, 点 A 移至 A' , 由图中几何关系知;

$$\Delta_{Cx} = \Delta_{Ax} = \Delta_{Ay} = 0.476 \text{mm}$$

$$\Delta_{Cy} = 0.476 \text{mm}$$



2-13 图示实心圆钢杆 AB 和 AC 在 A 点以铰相连接, 在 A 点作用有铅垂向下的力 $F=35\text{kN}$ 。已知杆 AB 和 AC 的直径分别为 $d_1=12\text{mm}$ 和 $d_2=15\text{mm}$, 钢的弹性模量 $E=210\text{GPa}$ 。试求 A 点在铅垂方向的位移。

解: 由节点 A 的平衡, $\sum F_x = 0$

$$F_{AB} \cos 45^\circ = F_{AC} \sin 30^\circ$$

$$\frac{\sqrt{2}}{2} F_{AB} = \frac{1}{2} F_{AC}, F_{AC} = \sqrt{2} F_{AB} \quad (1)$$

$$\sum F_y = 0, F_{AB} \cos 45^\circ + F_{AC} \cos 30^\circ = 35\text{kN}$$

$$\frac{\sqrt{2}}{2} F_{AB} + \frac{\sqrt{3}}{2} F_{AC} = 35\text{kN}$$

$$\sqrt{2} F_{AB} + \sqrt{3} F_{AC} = 70\text{kN} \quad (2)$$

联解 (1)、(2) 得

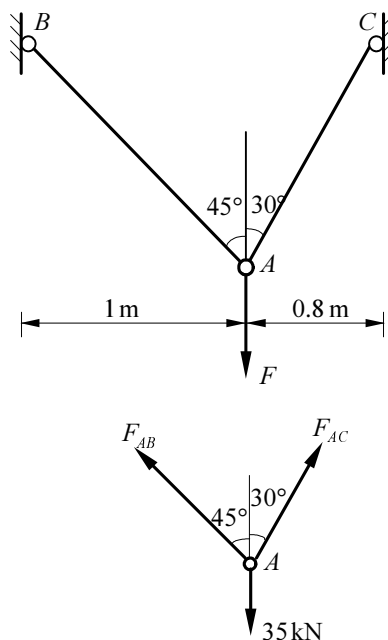
$$F_{NAB} = 18.2\text{kN}, F_{NAC} = 25.7\text{kN}$$

$$\Delta l_{AB} = \frac{F_{AB} l_{AB}}{EA_{AB}} = \frac{18.2 \times 10^3 \times \sqrt{2}}{210 \times 10^9 \times \frac{\pi \times 12 \times 12}{4} \times 10^{-6}}$$

$$= 1.08 \times 10^{-3} \text{m}$$

节点 A 的总位移为 $\overline{AA'}$

$$\frac{\Delta l_{AB}}{\overline{AA'}} = \cos \alpha_1 = \cos(45^\circ - \alpha)$$



$$\begin{aligned}\overline{AA'} &= \frac{\Delta l_{AB}}{\cos(45^\circ - \alpha)} \\ \frac{\Delta l_{AC}}{\overline{AA'}} &= \cos \alpha_2 = \cos(30^\circ + \alpha) \\ \overline{AA'} &= \frac{\Delta l_{AC}}{\cos(30^\circ + \alpha)} \\ \frac{\Delta l_{AB}}{\cos(45^\circ - \alpha)} &= \frac{\Delta l_{AC}}{\cos(30^\circ + \alpha)}\end{aligned}$$

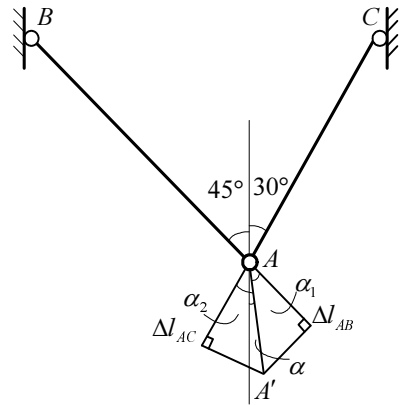
即

$$\frac{1.08}{0.707 \cos \alpha + 0.707 \sin \alpha} = \frac{1.1}{0.866 \cos \alpha - 0.5 \sin \alpha}$$

整理得: $0.157 \cos \alpha = 1.318 \sin \alpha$; $\tan \alpha = 0.119$, $\alpha = 6^\circ 47'$

故 A 点在垂直方向的位移 ΔA_\perp 为

$$\begin{aligned}\Delta A_\perp &= \overline{AA'} \cos \alpha = \frac{\Delta l_{AC}}{\cos(30^\circ + \alpha)} \cos \alpha = \frac{1.1 \times 10^{-3}}{\cos 36^\circ 47'} \times \cos 6^\circ 47' \\ &= \frac{1.1 \times 10^{-3} \times 0.993}{0.8009} = 1.365 \text{ mm}\end{aligned}$$



2-14 图示 A 和 B 两点之间原有水平方向的一根直径 $d = 1 \text{ mm}$ 的钢丝, 在钢丝的中点 C 加一竖直荷载 F 。已知钢丝产生的线应变为 $\varepsilon = 0.0035$, 其材料的弹性模量 $E = 210 \text{ GPa}$, 钢丝的自重不计。试求:

(1) 钢丝横截面上的应力 (假设钢丝经过冷拉, 在断裂前可认为符合胡克定律);

(2) 钢丝在 C 点下降的距离 Δ ;

(3) 荷载 F 的值。

解: (1) 求 σ

$$\sigma = E\varepsilon = 210 \times 10^9 \times 3.5 \times 10^{-5} = 735 \text{ MPa}$$

(2) 求钢丝在 C 点下降的距离 Δ

$$\varepsilon = \frac{\Delta_{AC}}{l_{AC}} = \frac{\Delta_{AC}}{1 \text{ m}}$$

$$\Delta_{AC} = \varepsilon \times 1 \text{ m} = 0.0035 \text{ m}$$

$$\text{伸长后 } AC \text{ 长: } l_{AC} = 1 + \Delta_{AC} = 1 + 0.0035 \text{ m} = 1.0035 \text{ m}$$

$$\text{即: } \sqrt{1^2 + \Delta^2} = 1.0035 \text{ m}$$

$$1 + \Delta^2 = (1.0035)^2 = 1.007 \text{ m}$$

$$\Delta^2 = 0.007 \text{ m}^2 \quad \Delta = 83.7 \text{ mm}$$

(3) 求此时荷载 F 的值

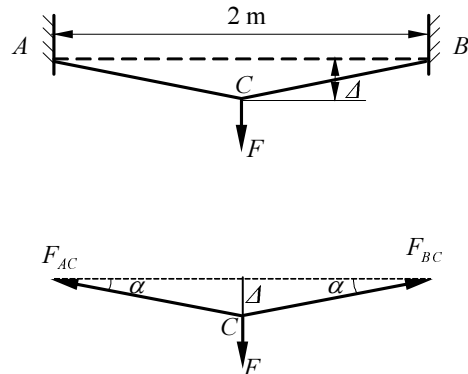
由节点 C 的力的平衡得:

$$2F_{AC} \sin \alpha = F$$

$$\text{即} \quad 2\sigma A \cdot \frac{0.837}{1.0035} = F$$

$$2 \times 735 \times 10^6 \times \frac{\pi \times 1^2}{4 \times 10^6} \times \frac{0.837}{1.0035} = F$$

$$F = 96.3 \text{ N}$$



2-15 图示圆锥形杆受轴向拉力作用，试求杆的伸长。

$$\text{解: } \Delta l = \int_0^l \frac{F dl}{EA(y)} = \frac{F}{E} \int_0^l \frac{dl}{A(y)}$$

取微段 dy 研究，将微段 dy 处的直径看成相同，则单

位长度的直径变化为： $\frac{d_2 - d_1}{l}$

故 dy 处的直径为： $d_1 + \frac{d_2 - d_1}{l}(l - y)$

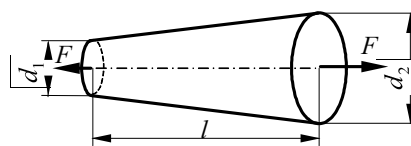
则 dy 处的面积为： $\frac{\pi}{4} [d_1 + \frac{d_2 - d_1}{l}(l - y)]^2$

$$\text{故 } \Delta l = \int_0^l \frac{F \cdot dy}{E \frac{\pi}{4} [d_1 + \frac{d_2 - d_1}{l}(l - y)]^2} = \frac{4Fl^2}{E\pi} \int_0^l \frac{dy}{[d_2 l - (d_2 - d_1)y]^2}$$

$$d[d_2 l - (d_2 - d_1)y] = 0 - (d_2 - d_1) dy$$

$$dy = \frac{d[d_2 l - (d_2 - d_1)y]}{-(d_2 - d_1)}$$

$$\begin{aligned} \text{故 } \Delta l &= \frac{-4Fl^2}{E\pi(d_2 - d_1)} \int_0^l \frac{d[d_2 l - (d_2 - d_1)y]}{[d_2 l - (d_2 - d_1)y]^2} = \frac{-4Fl^2}{E\pi(d_2 - d_1)} \left[\frac{-1}{d_2 l - (d_2 - d_1)y} \right]_0^l \\ &= \frac{4Fl^2}{E\pi(d_2 - d_1)} \left[\frac{1}{d_2 l - d_2 l + d_1 l} - \frac{1}{d_2 l} \right] = \frac{4Fl}{\pi d_1 \cdot d_2 E} \end{aligned}$$



2-16 有一长度为 300mm 的等截面钢杆承受轴向拉力 $F = 30 \text{ kN}$ 。已知杆的横截面面积 $A = 2500 \text{ mm}^2$ ，材料的弹性模量 $E = 210 \text{ GPa}$ 。试求杆中所积蓄的应变能。

$$\text{解: } V_\varepsilon = \frac{F_N^2 l}{2EA} = \frac{30 \times 30 \times 10^6 \times 0.3}{2 \times 210 \times 10^9 \times 2500 \times 10^{-6}} = 0.257 \text{ N} \cdot \text{m}$$

2-17 两根杆 $A_1 B_1$ 和 $A_2 B_2$ 的材料相同，其长度和横截面面积也相同。杆 $A B$ 承受作

用在端点的集中荷载 F ；杆 $A_2 B_2$ 承受沿杆长均匀分布的荷载，其集度为 $f = \frac{F}{l}$ 。试比较这

两根杆内积蓄的应变能。

解: $A B$ 杆在端部集中荷载 F 作用下， $V_{\varepsilon 1} = \frac{F^2 l}{2EA}$ ， $A_2 B_2$

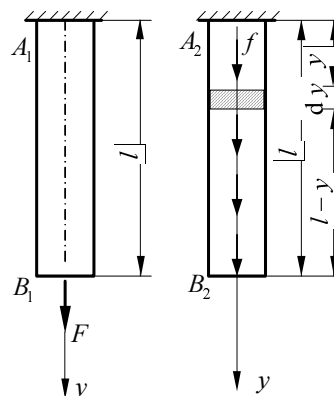
杆在沿杆长均匀分布的荷载作用下，轴力 F_N 沿 y 变化，则

dy 段杆上的 $F_N = \frac{F}{l}(l - y)$ 。

$$\text{故 } dV_{\varepsilon 2} = \frac{[\frac{F}{l}(l - y)]^2 dy}{2EA}$$

故

$$V_{\varepsilon 2} = \int_0^l \frac{[\frac{F}{l}(l - y)]^2 \cdot dy}{2EA} = \frac{F^2}{2EA l^2} \int_0^l (l^2 + y^2 - 2ly) dy$$



$$= \frac{F^2}{2EA l^2} \left[l^2 y + \frac{y^3}{3} - \frac{2ly^2}{2} \right] \Big|_0^l = \frac{F^2 l}{3 \times 2EA}$$

$$V_{\varepsilon 1} = 3V_{\varepsilon 2}$$

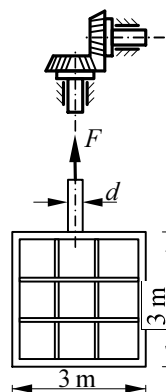
2-18 图示一钢筋混凝土平面闸门，其最大启门力为 $F = 140 \text{ kN}$ 。如提升闸门的钢质丝杆内径 $d = 40 \text{ mm}$ ，钢的许用应力 $[\sigma] = 170 \text{ MPa}$ ，试校核丝杠的强度

解： $\sigma = \frac{F}{A}$

$$= \frac{140 \times 10^3}{\frac{\pi \times 40 \times 40}{4} \times 10^{-6}}$$

$$= 111 \text{ MPa}$$

$\sigma < [\sigma]$ ，丝杠的强度够。

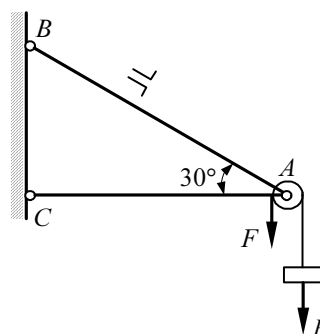


2-19 简易起重设备的计算简图如图所示。已知斜杆 AB 用两根 $63 \text{ mm} \times 40 \text{ mm} \times 4 \text{ mm}$ 不等边角钢组成，钢的许用应力 $[\sigma] = 170 \text{ MPa}$ 。试问在提起重量为 $P = 15 \text{ kN}$ 的重物时，斜杆 AB 是否满足强度条件？

解： $F_{NAB} \sin 30^\circ = 2W$

$$F_{NAB} = 4W = 4 \times 15 \text{ kN}$$

$$\sigma_{AB} = \frac{F_{NAB}}{A} = \frac{4 \times 15 \times 10^3}{2 \times 4.058 \times 10^{-4}} = 74 \text{ MPa}$$

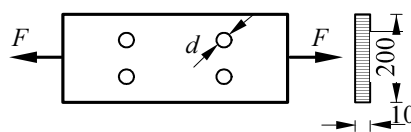


2-20 一块厚 10 mm 、宽 200 mm 的旧钢板，其截面被直径 $d = 20 \text{ mm}$ 的圆孔所削弱，圆孔的排列对称于杆的轴线，如图所示。钢板承受轴向拉力 $F = 200 \text{ kN}$ 。材料的许用应力 $[\sigma] = 170 \text{ MPa}$ ，试校核钢板的强度。

解： $\sigma = \frac{F}{(200 - 2 \times 20) \times 10 \times 10^{-6}}$

$$= \frac{200 \times 10^3 \times 10^6}{1600} = 125 \text{ MPa}$$

$\sigma < [\sigma]$ 强度够。



2-21 一结构受力如图所示，杆件 AB ， AD 均由两根等边角钢组成。已知材料的许用应力 $[\sigma] = 170 \text{ MPa}$ ，试选择杆 AB ， AD 的角钢型号。

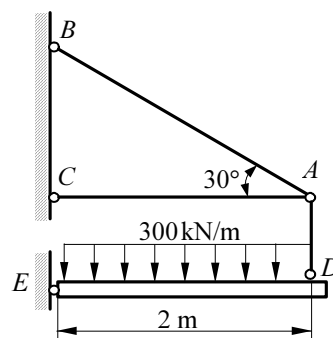
解：分离体图(a)

$$\sum M_E = 0, F_{NAD} \times 2 = 300 \times 10^3 \times \frac{1}{2} \times 2 \times 2$$

$$F_{NAD} = 300 \times 10^3 \text{ N}$$

由节点 A: $F_{NAB} \sin 30^\circ = F_{NAD}$

$$F_{NAB} = 2F_{NAD} = 600 \text{ kN}$$



$$\sum F_y = 0, \quad F_{FE} = 100 \times 3 + 60 - 174 = 186 \text{ kN}$$

$$\sigma_{AB} = \frac{F_{AB}}{2A_{AB}} \leq 170 \times 10^6$$

$$A_{AB} \geq \frac{240 \times 10^3}{2 \times 170 \times 10^6} = 7.059 \times 10^{-4} \text{ m}^2$$

故杆 AB 选用 2 根 $90 \times 56 \times 5$ 角钢, ($A = 2 \times 7.212 \text{ cm}^2$)

$$A_{CD} \geq \frac{F_{CD}}{2[\sigma]} = \frac{60 \times 10^3}{2 \times 170 \times 10^6} = 1.765 \times 10^{-4} \text{ m}^2$$

故杆 CD 选用 2 根 $40 \times 25 \times 3$ 角钢, ($A = 2 \times 1.89 \text{ cm}^2$)

$$A_{EF} = \frac{F_{EF}}{2[\sigma]} = \frac{186 \times 10^3}{2 \times 170 \times 10^6} = 5.47 \times 10^{-4} \text{ m}^2$$

故杆 EF 选用 2 根 $70 \times 45 \times 5$ 角钢, ($A = 2 \times 5.609 \text{ cm}^2$)

$$A_{HG} = \frac{F_{HG}}{2[\sigma]} = \frac{174 \times 10^3}{2 \times 170 \times 10^6} = 5.12 \times 10^{-4} \text{ m}^2$$

故杆 HG 选用 2 根 $70 \times 45 \times 5$ 角钢。

$$\Delta_G = \frac{F_{HG} l_3}{A_{HG} E} = \frac{174 \times 10^3 \times 2}{2 \times 5.609 \times 10^{-4} \times 210 \times 10^9} = 1.477 \times 10^{-3} \text{ m} = 1.477 \text{ mm}$$

$$\Delta_E = \frac{F_{EF}}{F_{HG}} \cdot \Delta_G = \frac{186}{174} \times 1.477 \text{ mm} = 1.579 \text{ mm}$$

$$\text{故 } \Delta_D = 1.477 + \frac{1.8}{3} \times (1.579 - 1.477) = 1.54 \text{ mm}$$

$$\Delta_C = \Delta_D + \frac{F_{CD} l_2}{EA_{CD}} = 1.54 \times 10^{-3} + \frac{60 \times 10^3 \times 1.2}{2 \times 1.89 \times 10^{-4} \times 210 \times 10^9}$$

$$= 2.45 \times 10^{-3} \text{ m} = 2.45 \text{ mm}$$

$$\Delta_A = \frac{F_{AB} l_1}{EA_{AB}} = \frac{240 \times 10^3 \times 3.4}{2 \times 7.26 \times 10^{-4} \times 210 \times 10^9} = 2.68 \times 10^{-3} = 2.68 \text{ mm}$$

2-24 已知混凝土的密度 $\rho = 2.25 \times 10^3 \text{ kg/m}^3$, 许用压应力 $[\sigma] = 2 \text{ MPa}$ 。试按强度条件确定图示混凝土柱所需的横截面面积 A_1 和 A_2 。若混凝土的弹性模量 $E = 20 \text{ GPa}$, 试求柱顶 A 的位移。

解: 上部截面 C 轴力最大。

$$F_{1\max} = 1000 \times 10^3 + \rho g A_1 \times 12$$

$$= 1000 \times 10^3 + 2.25 \times 9.8 \times 10^3 A_1 \times 12$$

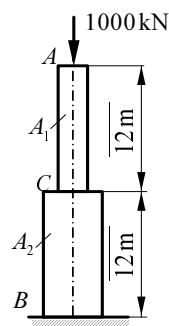
$$\sigma_{\max} = \frac{1 \times 10^6}{A_1} + 22 \times 12 \times 10^3 \leq 2 \times 10^6$$

$$\text{故 } A_1 \geq \frac{1 \times 10^6}{10^6 (2 - 2.2 \times 1.2 \times 10^{-1})} = 0.576 \text{ m}^2$$

$$F_{2\max} = 1000 \times 10^3 + 22 \times A_1 \times 12 \times 10^9 + 22 \times A_2 \times 12 \times 10^3$$

$$= 1152 \times 10^3 + 264 \times 10^3 A_2$$

$$\sigma_{2\max} = \frac{1152 \times 10^3 + 264 \times 10^3 A_2}{A_2} \leq 2 \times 10^6$$



$$\text{故 } A_2 \geq \frac{1152 \times 10^3}{1.736 \times 10^6} = 0.665 \text{ m}^2$$

$$\begin{aligned} \Delta_A &= \frac{1000 \times 10^3 \times 12}{EA_1} + \int_0^{12} \frac{(\rho g A_1 y) dy}{EA_1} + \frac{(1000 \times 10^3 + \rho g A_1 \times 12) \times 12}{EA_2} + \int_0^{12} \frac{\rho g A_2 y dy}{EA_2} \\ &= \frac{1}{E} \left[\frac{1 \times 10^6 \times 12}{0.576} + \frac{\rho g l^2}{2} \right]_0^{12} + \frac{(1 \times 10^6 + 22 \times 10^3 \times 0.576 \times 12) \times 12}{0.665} + \frac{\rho g l^2}{2} \Big|_0^{12} \\ &= \frac{1}{20 \times 10^9} \left[\frac{1 \times 10^6 \times 12}{0.576} + 22 \times 10^3 \times 12^2 + \frac{11.52 \times 10^5 \times 12}{0.665} \right] = 2.24 \text{ mm} \end{aligned}$$

2-25 (1) 刚性梁 AB 用两根钢杆 AC , BD 悬挂, 其受力如图所示。已知钢杆 AC 和 BD 的直径分别为 $d_1 = 25 \text{ mm}$ 和 $d_2 = 18 \text{ mm}$, 钢的许用应力 $[\sigma] = 170 \text{ MPa}$, 弹性模量 $E = 210 \text{ GPa}$ 。试校核钢杆的强度, 并计算钢杆的变形 Δl_{AC} , Δl_{DB} 及 A , B 两点的铅直位移 Δ_A , Δ_B 。

(2) 若荷载 $F = 100 \text{ kN}$ 作用于 A 点处, 试求 F 点的铅直位移 Δ_F 。(计算结果表明, $\Delta_F = \Delta$, 事实上这是线性弹性体中普遍存在的关系, 称为位移互等定理。)

解: (1) $F_D = 33.3 \text{ kN}$, $F_{AC} = 66.7 \text{ kN}$

$$\sigma_{AC} = \frac{66.7 \times 10^3}{\frac{\pi \times 25^2 \times 10^{-6}}{4}} = 136 \text{ MPa} < [\sigma] \quad \text{安全}$$

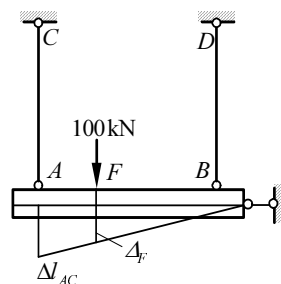
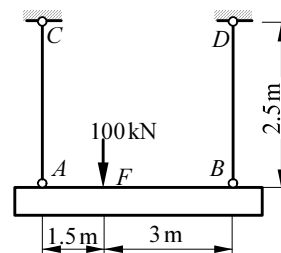
$$\sigma_{DB} = \frac{33.3 \times 10^3}{\frac{\pi \times 18^2 \times 10^{-6}}{4}} = 131 \text{ MPa} < [\sigma] \quad \text{安全}$$

$$\Delta l_{AC} = \frac{136 \times 10^6 \times 2.5}{210 \times 10^9} = 1.62 \text{ mm}$$

$$\Delta l_{DB} = \frac{131 \times 10^6 \times 2.5}{210 \times 10^9} = 1.56 \text{ mm}$$

$$(2) \Delta l_{AC} = \frac{100 \times 10^3 \times 2.5}{210 \times 10^9 \times \frac{\pi \times 25^2 \times 10^{-6}}{4}} = 2.43 \text{ mm}$$

$$\Delta_F = \frac{3 \times 2.43}{4.5} = 1.62 \text{ mm}$$



2-26 图示三铰拱屋架的拉杆用 16 锰钢杆制成。已知此材料的许用应力 $[\sigma] = 210 \text{ MPa}$, 弹性模量 $E = 210 \text{ GPa}$ 。试按强度条件选择钢杆的直径, 并计算钢杆的伸长。

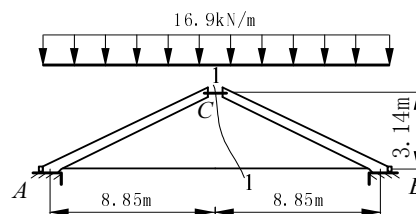
解: $F_{RA} = F_{RB} = 149 \text{ kN}$

$$\overline{AC} = \sqrt{3.14^2 + 8.85^2} = 9.4 \text{ m}$$

取 1-1 以左分离体: $\sum M_C = 0$

$$16.9 \times \frac{8.85^2}{2} + 3.14 F_{AB} - 149 \times 8.85 = 0$$

$$F_{AB} = 210 \text{ kN}$$



$$\sigma_s = \frac{F_{AB}}{A} = \frac{210 \times 10^3}{\frac{\pi d^2}{4} \times 10^{-6}} \leq 210 \times 10^6$$

$$d^2 \geq 1270, \quad d \geq 35.6 \text{ mm}$$

$$\Delta l_s = \sigma_s \times \frac{l_{AB}}{E} = 210 \times 10^6 \times \frac{17.7}{210 \times 10^9} = 17.7 \times 10^{-3} \text{ m}$$

故 $\Delta l_s = 17.7 \text{ mm}$

2-27 简单桁架及其受力如图所示，水平杆 BC 的长度 l 保持不变，斜杆 AB 的长度可随夹角 θ 的变化而改变。两杆由同一材料制造，且材料的许用拉应力与许用压应力相等。要求两杆内的应力同时达到许用应力，且结构的总重量为最小时，试求：

- (1) 两杆的夹角 θ 值；
- (2) 两杆横截面面积的比值。

解：(1) 各杆轴力，图 (a)

$$\sum F_y = 0, F_1 \sin \theta - F = 0, F_1 = \frac{F}{\sin \theta}$$

$$\sum F_x = 0, F_1 \cos \theta + F_2 = 0, F_2 = -\frac{F \cos \theta}{\sin \theta}$$

- (2) 两杆同时达到许用应力时的横截面面积

$$A_1 = \frac{F_1}{[\sigma]} = \frac{F}{\sin \theta [\sigma]}, A_2 = \frac{F_2}{[\sigma]} = \frac{F \cos \theta}{\sin \theta [\sigma]}$$

- (3) 结构具有最小重量时的 θ 值，结构的总重量：

$$W = W_1 + W_2 = \rho g A_1 l_1 + \rho g A_2 l_2 = \frac{\rho g F l}{[\sigma]} \left(\frac{1}{\sin \theta \cos \theta} + \frac{\cos \theta}{\sin \theta} \right)$$

$$\frac{dW}{d\theta} = \frac{\rho g F l}{[\sigma]} \left(\frac{\sin^2 \theta - \cos^2 \theta}{\sin^2 \theta \cos^2 \theta} - \frac{\sin^2 \theta + \cos^2 \theta}{\sin^2 \theta} \right) = 0$$

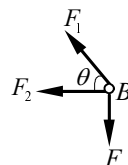
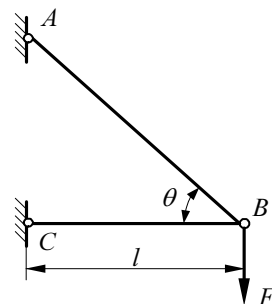
$$\tan^2 \theta - 2 = 0$$

$$\theta = \arctan \sqrt{2} = 54^\circ 44'$$

- (4) 两杆横截面面积之比

许用拉应力与许用压应力相等，故横截面面积之比等于其轴力之比，即：

$$\frac{A_{AB}}{A_{BC}} = \frac{A_1}{A_2} = \frac{F_1}{F_2} = \frac{1}{\cos \theta} = \sqrt{3}$$



2-28 一内半径为 r ，厚度为 δ ($\delta \leq \frac{r}{10}$)，宽度为 b 的薄壁圆环。在

圆环的内表面承受均匀分布的压力 p (如图)，试求：

- (1) 由内压力引起的圆环径向截面上的应力；
- (2) 由内压力引起的圆环半径的伸长。

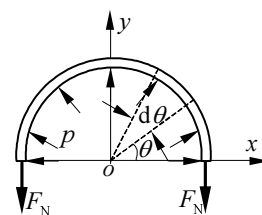
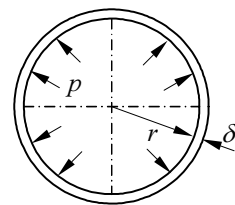
解：(1) 径向截面上的内力

用截面法得受力图 (a)。任取微弧段 $r d\theta$ ，作用在该微段上的径向力为

$$dF = p \cdot b r d\theta$$

由 $\sum F_y = 0$ ，得

$$2F_N = \int_0^\pi p b r d\theta \cdot \sin \theta = 2 p b r, \quad F_N = p b r$$



(2) 径向截面上应力

$$\sigma = \frac{F_N}{A} = \frac{pbr}{\delta b} = \frac{pr}{\delta}$$

(3) 圆环可看成是宽度为 b ，厚度为 δ ，长度为 $2\pi(r + \frac{\delta}{2})$ ，并受轴力 F_N 的平板条，因此圆环沿圆长方向的伸展为：

$$\delta_c = \frac{F_N l}{EA} = \frac{pbr \cdot 2\pi(r + \frac{\delta}{2})}{E \cdot b \delta} = \frac{2\pi pr^2}{E\delta} (1 + \frac{\delta}{2r})$$

半径伸长：

$$\delta_r = \frac{\delta_c}{2\pi} = \frac{pr^2}{E\delta} (1 + \frac{\delta}{2r})$$

因是薄壁圆环，故 $\frac{\delta}{2r} \ll 1$ ，得

$$\delta_r = \frac{pr^2}{E\delta}$$